

MECHANICAL PROPERTIES OF ALUMINIUM BRACKET STRENGTHENING

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The aim of the research is laboratory investigation of aluminium brackets employed to fasten lightweight curtain walls to building facilities. Tensile loads perpendicular to end plates (vertical) were applied here. The author focused on the solutions intended to increase the load-carrying capacity of aluminium brackets applying the plain washer form A (DIN 125; ISO 7089), plain washer with an outer diameter about 3d (DIN 9021; ISO 7093) and additional cover plates (straps) in the location of bolt anchoring on the base plate. The aluminium brackets were tested on a steel base and concrete substrate. The flexibility of anchoring strongly affects the increase of the end plate middle point displacement and movable crosshead displacement.

Keywords: aluminium bracket, cover plates, mechanical properties, EN AW-6060 T66

1. INTRODUCTION

Aluminium brackets form a group of the most popular elements applied to fasten lightweight curtain walls to building facilities in civil engineering (e.g. in mullion-transom wall systems to connect box-section aluminium profiles to structures, see Fig. 1). Curtain walls are complex systems composed of separate components (see e.g. Kazmierczak [8]). One of the most important design problems of curtain walls is a proper choice of a bracket and its fastening to a building structure. Improper bracket design and/or its fastening may trigger problems with curtain walls (see e.g. Tovarović et al. [9]). Moreover, inadequate support of glass façade elements eventually brings about the extreme danger to life and property. Ambroziak et al. [1] have undertaken numerical and analytical investigations of aluminium bracket strengthening by additional cover plates (straps) in

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the case of insufficient load-bearing capacity and stiffness due to design loads. The proposed research addresses laboratory tests to experimentally assess the increase of bearing capacity of the aluminium bracket applied to fasten lightweight curtain walls to building facilities.



Fig. 1. View on fixed aluminium brackets in mullion-transom wall system

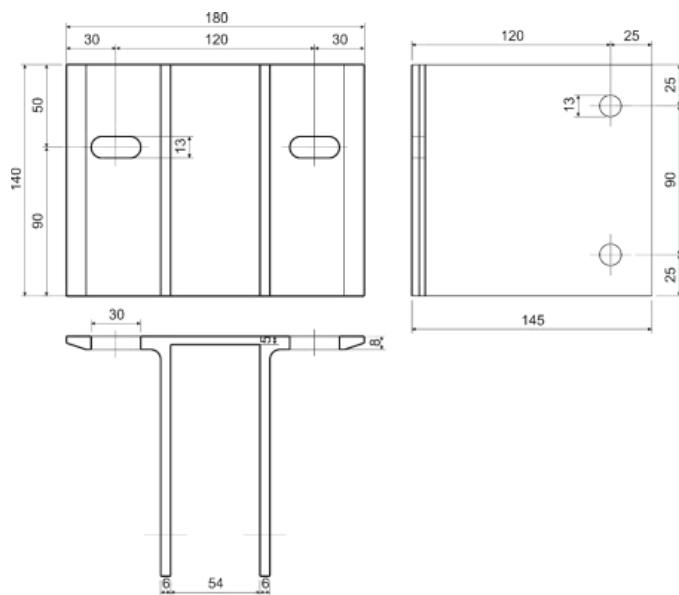


Fig. 2. View and cross-section of analysed aluminium bracket

2. MATERIALS

An investigated aluminium bracket consists of an end plate and two cantilever plates (Fig. 2). The end plate is 180 mm wide and 140 mm high, showing various thickness: 8 mm and 5 mm (in the middle of the end plate). The cantilever plate of 6 mm thickness is 145 mm wide and 140 mm high, includes two normal holes of a 13 mm diameter. Two oval holes (13 mm by 30 mm, Fig. 2) in the end plate are provided to fasten the aluminium bracket to the building structure.

Table 1. Mechanical properties of EN AW-6060 T66 according to EN 755-2 [5]

properties	values
Proof stress $R_{e0.2}$ [MPa]	150
Tensile strength R_m [MPa]	195
Elongation min.	8
Elongation max.	8
Brinell hardness	65
n -value in Ramberg-Osgood expression for plastic analysis	18

Table 2. Technical data of a single anchor in non-cracked normal concrete

properties	values
Nominal drill hole diameter d_0 [mm]	12
Min. member thickness h_{min} [mm]	120
Effective anchorage depth h_{ef} [mm]	70
Installation torque T_{inst} [Nm]	60
Steel failure – characteristic resistance $N_{Rk,s}$ [kN]	44.3
Pullout failure – characteristic resistance in uncracked concrete C30/37 $N_{Rk,p}$ [kN]	26.8
Displacement – factor for tensile load δ_{Nz} [mm/kN]	0.06-0.14

In laboratory tests two variants of bases (steel plate and concrete foundation) were applied, see Fig. 3. In the first variant, two A4-70 stainless steel bolts M12 (diameter of 12 mm) were used to fasten the aluminium bracket to the steel plate of 25 mm thickness. In the concrete foundation case two bolt anchors made of A4 stainless steel were applied. The anchoring process of the aluminium bracket on concrete consists of several actions. First of all the drill hole was made, next, the anchor was hammered in, finally, the nut was tightened with a controlled torque equal 60Nm (Table 2). Reinforced concrete elements were applied of strength class C30/37 (compressive strength: 30MPa cylinder and 37 cube according to standard EN206 [7]), their dimensions are: thickness 20 cm, width 38 cm and length 70 cm. The laboratory tests were focused on solutions to increase the load-carrying capacity of aluminium brackets due to three types of washers applied in the location of bolt anchoring on an end plate, see Fig. 2. The applications were tested of the A4 stainless steel flat

washer form A (DIN 125; ISO 7089), A4 stainless steel flat washer with an outer diameter about $3d$ (DIN 9021; ISO 7093, called large washer) and additional aluminium cover plates (straps), see Fig. 4. The laboratory-tested concept of a strengthened end plate of the aluminium bracket by additional cover plates (straps) can be compared with the concept of a reinforced T-stub flange with backing plates (see e.g. Zoetemeijer [3], Nair et al. [4]). The aluminium bracket and cover plates (straps) are made of EN AW-6060 T66 aluminium alloy. The material parameters for the EN AW-6060 T66 aluminium alloy can be found in civil engineering standards. Table 1 includes major mechanical properties of the EN AW-6060 T66 aluminium alloy due to wall thickness from 3 to 25 mm. In the case of concrete foundation, the M12 bolt anchors were installed in a drilled hole in a concrete element and anchored by torque-controlled expansion. The base technical data due to torque-controlled expansion anchor dedicated to concrete is available in Table 2.

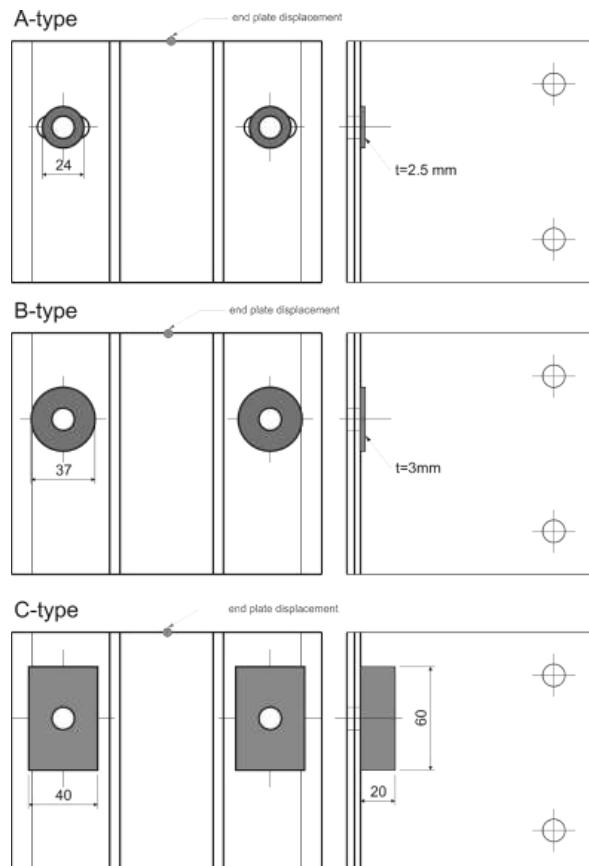


Fig. 3. Analysed aluminium bracket with plain washers and cover plates (straps)



Fig. 4. Laboratory test stands for steel and concrete bases



Fig. 5. Damaged aluminium brackets after tensile tests

3. LABORATORY TESTS

The tensile loads perpendicular to the end plate (vertical) were applied in laboratory tests by means of the computer-controlled Zwick testing machine (Fig. 4). The aluminium brackets were subjected to tension with a constant loading speed (force controlled) equal 5000 N/min (according to the ETAG 034 guideline [6]. All tests were performed at room temperature (about 20°C). First of all, the experiments were performed with a constant loading speed to failure of the aluminium brackets or damage of anchoring under the steel base (type A1, B1, and C1) and concrete base (type A2, B2, and C2), see Fig. 5. The diagram of tensile force vs displacement of movable crosshead up to rupture of the aluminium bracket is shown in Fig. 6. The values of rupture forces obtained in the aluminium bracket laboratory tests are shown in Table 3. It should be noted, that in the case of steel

base the possibilities are damage of the end plate (see Fig. 5a) or failure of steel bolts. On the other hand, in the case of concrete foundations, rupture forces are determined by the strength of bolt anchoring (without cracks and tears of aluminium bracket), see Fig. 5b.

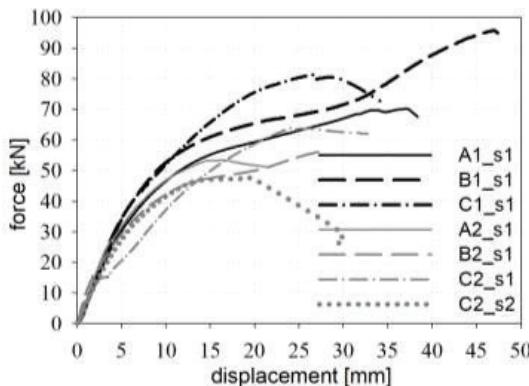


Fig. 6. Tensile force versus displacement up to rupture of aluminium bracket

Table 3. Rupture forces [kN] of aluminium bracket

steel base	value	concrete base	value
A1_s1	70.3	A2_s1	53.4
B1_s1	95.8	B2_s1	56.2
C1_s1	81.4	C2_s1_s2mean	55.8

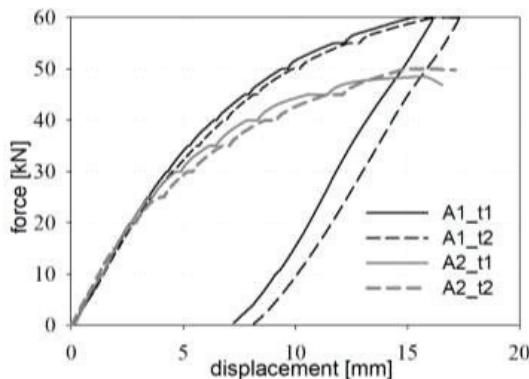


Fig. 7. Tensile force versus displacement – A type tests

Next, the force-controlled tests were carried out up to 60kN due to the steel base and up to 50kN due to the concrete base. The force held every 5kN in its waiting time equal to 120s. When the force reached the monitoring value (5kN, 10kN, etc.), displacement of the end plate (a point in Fig. 4)

was measured. Displacement measurement of the middle point of the end plate was carried out at the start of a specific load level. When the force reached 60kN in a steel base and 50kN in a concrete foundation the aluminium brackets were unloaded after a specified waiting time. All C-type specimens fulfilled the testing conditions, due to A and B types a single test leads to an end only.

Figures 7-9 show the diagrams of tensile force vs displacement of movable crosshead due to A, B, and C type tests. Displacements of movable crosshead (crosshead travel) under specific load are collected in Tables 4 and 5 for steel base and concrete foundation, respectively. Additionally, the displacement of the end plate middle point is shown in Tables 6 and 7 and in Figures 10-12.

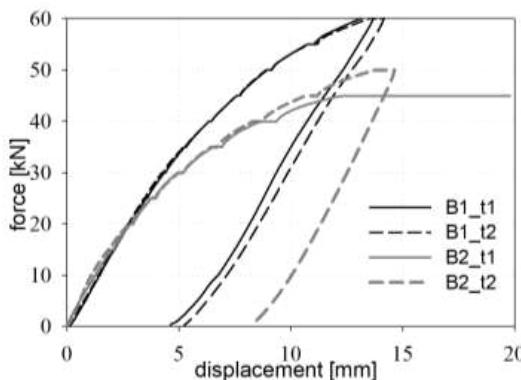


Fig. 8. Tensile force versus displacement – B type tests

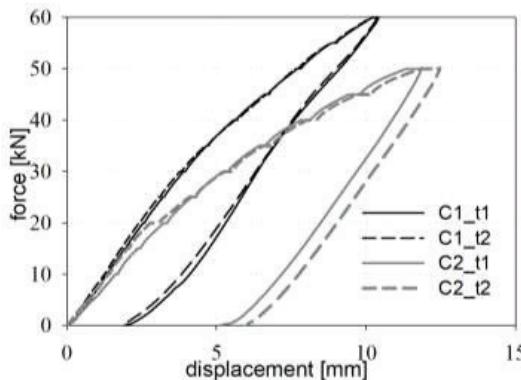


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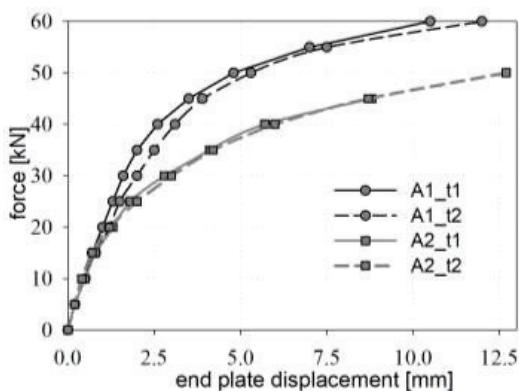


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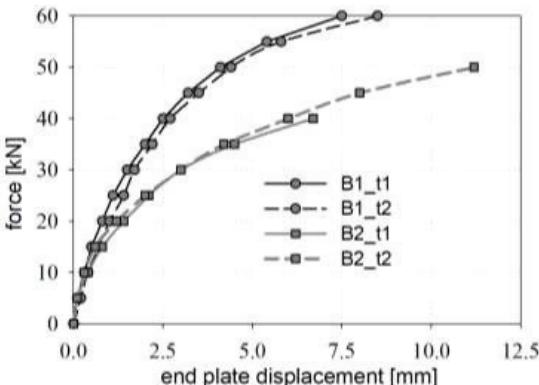


Fig. 11. Tensile force versus end plate displacement – B type tests

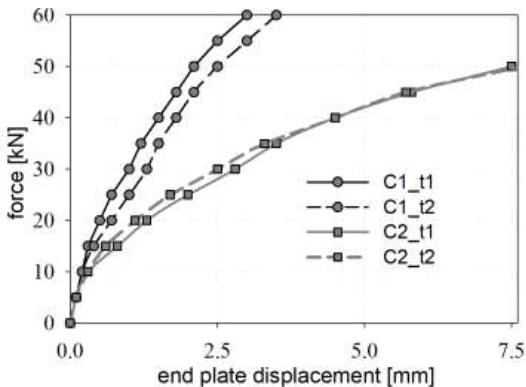


Fig. 12. Tensile force versus end plate displacement – C type tests

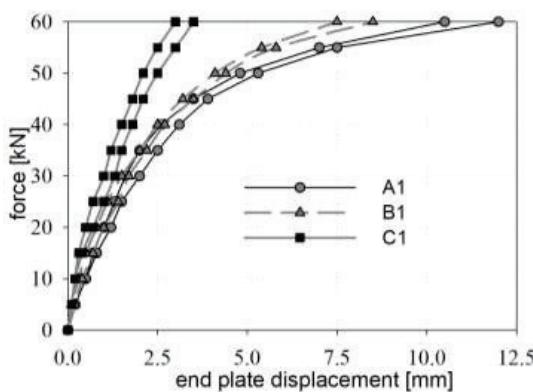


Fig. 13. Tensile force versus end plate displacement – steel base

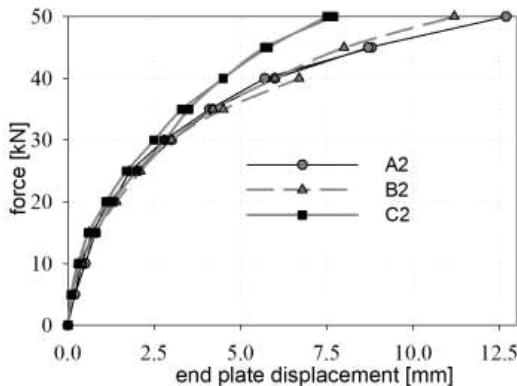


Fig. 14. Tensile force versus end plate displacement – concrete foundation

Table 4. Displacement [mm] of movable crosshead under specific load – steel base

force [kN]	A1_t1	A1_t2	A1mean	B1_t1	B1_t2	B1mean	C1_t1	C1_t2	C1mean
5	0.78	0.79	0.78	0.83	0.77	0.80	0.79	0.65	0.72
10	1.41	1.48	1.44	1.47	1.43	1.45	1.36	1.27	1.31
15	2.03	2.13	2.08	2.14	2.09	2.11	1.96	1.83	1.89
20	2.71	2.85	2.78	2.84	2.77	2.80	2.59	2.45	2.52
25	3.47	3.65	3.56	3.59	3.49	3.54	3.25	3.11	3.18
30	4.33	4.61	4.47	4.41	4.31	4.36	3.96	3.84	3.90
35	5.30	5.71	5.50	5.34	5.27	5.30	4.73	4.69	4.71
40	6.49	6.94	6.71	6.46	6.46	6.46	5.66	5.70	5.68
45	7.92	8.32	8.12	7.74	7.71	7.72	6.71	6.75	6.73
50	9.66	10.19	9.92	9.15	9.12	9.13	7.83	7.74	7.78
55	12.19	12.86	12.52	11.05	11.18	11.11	9.01	8.96	8.98
60	16.15	17.31	16.73	13.67	14.63	14.15	10.42	10.37	10.39

Table 5. Displacement [mm] of movable crosshead under specific load – concrete base

Force [kN]	A2_t1	A2_t2	A2mean	B2_t1	B2_t2	B2mean	C2_t1	C2_t2	C2mean
5	0.71	0.64	0.67	0.59	0.58	0.58	0.89	0.74	0.81
10	1.43	1.26	1.34	1.29	1.12	1.20	1.69	1.41	1.55
15	2.02	1.98	2.00	2.09	1.92	2.00	2.45	2.07	2.26
20	2.71	2.89	2.80	2.97	2.89	2.93	3.35	3.08	3.21
25	3.65	4.15	3.90	3.93	3.96	3.94	4.31	4.28	4.29
30	4.91	5.44	5.17	5.22	5.21	5.21	5.39	5.51	5.45
35	6.47	7.06	6.76	6.93	6.73	6.83	6.65	6.84	6.74
40	8.32	9.16	8.74	9.38	8.73	9.05	8.14	8.34	8.24
45	11.46	12.13	11.79	21.0D3	11.20	-	9.80	10.16	9.98
50	19.1D1	20.8D2	-		14.63	-	11.85	12.46	12.15

Rupture forces: D1=48.45kN; D2=50kN; D3=45.07kN

Table 6. Displacement [mm] of end plate middle point under specific load – steel base

force [kN]	A1_t1	A1_t2	A1mean	B1_t1	B1_t2	B1mean	C1_t1	C1_t2	C1mean
5	0.2	0.2	0.2	0.1	0.2	0.15	0.1	0.1	0.1
10	0.5	0.5	0.5	0.3	0.4	0.35	0.2	0.2	0.2
15	0.7	0.8	0.75	0.5	0.7	0.6	0.3	0.4	0.35
20	1.0	1.2	1.1	0.8	1.0	0.9	0.5	0.7	0.6
25	1.3	1.5	1.4	1.1	1.4	1.25	0.7	1.0	0.85
30	1.6	2.0	1.8	1.5	1.7	1.6	1.0	1.3	1.15
35	2.0	2.5	2.25	2.0	2.2	2.1	1.2	1.5	1.35
40	2.6	3.1	2.85	2.5	2.7	2.6	1.5	1.8	1.65
45	3.5	3.9	3.7	3.2	3.5	3.35	1.8	2.1	1.95
50	4.8	5.3	5.05	4.1	4.4	4.25	2.1	2.5	2.3
55	7.0	7.5	7.25	5.4	5.8	5.6	2.5	3.0	2.75
60	10.5	12.0	11.25	7.4	8.5	7.95	3.0	3.5	3.25

Table 7. Displacement [mm] of end plate middle point under specific load – concrete base

Force [kN]	A2_t1	A2_t2	A2mean	B2_t1	B2_t2	B2mean	C2_t1	C2_t2	C2mean
5	0.2	0.2	0.2	0.1	0.1	0.1	0.1	0.1	0.1
10	0.5	0.4	0.45	0.4	0.3	0.35	0.3	0.3	0.3
15	0.8	0.7	0.75	0.8	0.6	0.7	0.8	0.6	0.7
20	1.3	1.2	1.25	1.4	1.2	1.3	1.3	1.1	1.2
25	1.8	2.0	1.9	2.1	2.0	2.05	2.0	1.7	1.85
30	2.8	3.0	2.9	3.0	3.0	3.0	2.8	2.5	2.65
35	4.1	4.2	4.15	4.5	4.2	4.35	3.5	3.3	3.45
40	5.7	6.0	5.85	6.7	6.0	6.35	4.5	4.5	4.5
45	8.8	8.7	8.75	-D3	8.0	-	5.8	5.7	5.75
50	-D1	12.7D2	-		11.2	-	7.5	7.7	7.6

3. DISCUSSION AND CONCLUSIONS

The investigation is aimed at assessing mechanical properties of aluminium brackets strengthened by washers and straps (cover plates) on steel base and concrete foundations. The paper supplements and develops former investigations carried out by Ambroziak et al. [1], it may provide scientists, engineers, and designers with an experimental and theoretical basis in the field of mechanical properties of strengthened aluminium brackets. Based on the performed laboratory tests the following conclusions may be drawn:

- Rupture forces of the aluminium bracket on a steel base (see Fig. 6 and Table 3) rise up (36% and 15%) while applying the washer with an outer diameter about 3d (large washer) and additional aluminium cover plates (straps). On the other hand, while an aluminium bracket is tested on a concrete foundation (see Fig. 6 and Table 3) insignificant increase of rupture forces (about 5%) is observed. Rupture forces are determined by means of bearing capacity of bolt anchors. It should be noted that permissible loads for bolt anchoring are affected by a great number of factors (see e.g. Ambroziak and Solarczyk [2]).
- The displacements of the end plate middle point of the aluminium bracket with cover plates (C type) under tensile loads on a steel plate (see Fig. 13 and table 6) clearly exhibit the increase in stiffness and decrease in displacements, comparing the A and B type specimens. On the other hand, when aluminium brackets on concrete foundations were tested the differences were apparent in the case of higher loads (see Fig. 14 and table 7). The aluminium bracket with cover plates (C type) shows an increase in stiffness and decrease in displacement over the tensile load of 25kN.
- The flexibility of anchoring strongly affects the increase of the end plate middle point displacement and movable crosshead displacement (compare Figs. 7-12). It should be noted that flexibility of anchoring on a steel base is restrained by stretching the steel bolts only (see Fig. 5). When the torque-controlled expansion anchors are applied additional slip in the anchoring is observed. The bolt anchor manufacturers' guidelines specify a displacement value of bolt anchor under tensile load (see table 2, factor $\delta_{N\infty}$ mm/kN). The design of bolt anchoring should be intended not only to fulfil loading conditions but also to compare their flexibility (slip in anchoring due to loads).
- Laboratory tests on rigid metal profiles (steel plates) indicate different values of the middle point displacement and movable crosshead displacement. The guidelines for determining the

resistance of aluminium brackets on rigid metal profile (see e.g. the ETAG 034 guideline [6]) should be verified. The laboratory tests should be carried out on a concrete substrate with specified anchoring systems to determine proper mechanical behaviour of aluminium brackets.

The paper is dedicated to the engineers performing structural design of aluminium brackets to fasten lightweight curtain walls to building facilities. It provides the information on mechanical properties under vertical loads of aluminium brackets and the possibilities of its strengthening. The results encourage the author to continue the research, on the basis of performing laboratory tests with biaxial loads (vertical and horizontal), complemented by cyclic and rheological tests in order to assess the long-time properties of aluminium brackets strengthened by additional aluminium cover plates (straps) or stainless steel large washer.

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WŁAŚCIWOŚCI MECHANICZNE WZMOCNIONYCH KONSOL ALUMINIOWYCH

Keywords: konsole aluminiowe, właściwości mechaniczne, EN AW-6060 T66, wzmacnianie konsoli

STRESZCZENIE:

Przedmiotem przedstawionych w artykule badań są konsole aluminiowe, które są stosowane jako łączniki fasad szupowo-ryglowych z konstrukcją budynku. Konsole wykonano ze stopu aluminium AW-6060 wg PN-EN 573-3:2004, odmiana T66 wg PN-EN 515:1996. W pracy badano wpływ zastosowania w miejscu kotwieni konsol aluminiowych zwykłej stalowej podkładki (typ A), powiększonej stalowej podkładki (typ B) oraz aluminiowych nakładek (tzw. stempli, typ C). Konsole aluminiowe kotwione do podłożą stalowego i betonowego wykorzystując śruby lub kotwy mechaniczne. Zastosowanie dwóch typów podłożą, do którego kotwiono konsole aluminiowe, umożliwiło ocenę wpływów podatności kotwienia mechanicznego na nośność i sztywność wzmacnianych konsol aluminiowych. Wyniki badań wskazują iż zastosowanie aluminiowych nakładek, usytuowanych pod nakrętkami śrub łączących je z konstrukcją budynku, zwiększa sztywności konsol aluminiowych. Nakładek wzmacniają lokalnie blachy czołowe konsoli. Ich zadaniem konstrukcyjnym jest m.in. zmniejszenie ramienia „zginania” *c* oraz zwiększenie sztywności giętej blachy czołowej konsoli.

Artykuł uzupełnia i rozwija badania prowadzone przez Ambroziaka i innych [1]. Wykonane i opisane badania stanowią podstawę do szczegółowego opisania właściwości mechanicznych testowanych rozwiązań wzmacniania konsol aluminiowych. Nośność konsoli aluminiowej zamocowanej do stalowej płyty wzrasta (od 15% do 36%) po zastosowaniu podkładek powiększonych i dodatkowych nakładek aluminiowych. W przypadku kotwienia konsoli aluminiowej do zbrojonego elementu betonowego obserwuje się nieznaczny wzrost nośności (około 5%). W tym przypadku nośność konsoli aluminiowej determinowana jest nośnością zastosowanego kotwienia mechanicznego. Przemieszczenia środkowego punktu blachy czołowej konsoli aluminiowej z nakładkami (typu C) zamocowanej do podłożą stalowego pod obciążeniem rozciągającym wykazuje wzrost sztywności i zmniejszenie przemieszczeń w porównaniu z typami A i B. Dla konsol aluminiowych kotwionych do podłożą betonowego efekt zwiększenia sztywności z zastosowaniem nakładek widoczny jest w przypadku wyższych wartości obciążień.

Podatność kotwienia silnie wpływa na zwiększenie przemieszczeń konsoli aluminiowej (w tym punktu środkowego blachy czołowej) i utratę nośności konsoli. Podatność zamocowania konsoli do stalowej płyty ograniczona jest do wydłużenia się stalowych śrub. W przypadku zastosowania kotew rozprężnych z kontrolowanymi momentem dokręcenia obserwuje się dodatkowy ich poślizg w zakotwieniu. Wytyczne techniczne producentów kotew określają wartość tych przemieszczeń. Prawidłowy proces projektowania zakotwienia mechanicznego w przypadku konsoli nie powinien ograniczać się jedynie do spełnienia wymagań nośności łączników, ale także do ograniczenia ich podatności (ograniczenia poślizgu w kotwieniu pod wpływem obciążień). Do prawidłowej oceny nośności sztywności konsol aluminiowych badania mechaniczne należy prowadzić na podłożu betonowym z zastosowaniem projektowanego systemu kotwienia.

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